

BERRY SURVEYING & ENGINEERING

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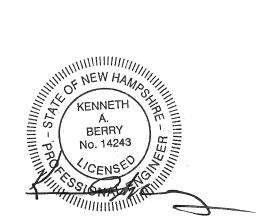
April 9, 2020

Town of Nottingham Planning Office Attention: JoAnna Arendarczyk, Planning Secretary 139 Stage Road P.O. Box 114 Nottingham, NH 03290

RE: Traffic Impact Analysis & Distribution

Robert L. DiBerto Mitchell Road Tax Map 242, Lot 1N Nottingham, NH 03290

Ms. Arendarczyk,



Pursuant to the Town of Nottingham Subdivision Regulations, Berry Surveying & Engineering (BS&E), on behalf of Robert L. DiBerto, has prepared a Standard Traffic Impact Analysis for the development of fourteen residential units on Tax Map 7, Lot 1N. The three points of analysis are the two intersections of Stone Grey Drive and Mitchell Road and a shared driveway and Mitchell Road.

The following conclusions were reached as a result Traffic Impact Analysis:

- A total of 4 vehicle trips (1 enter/3 exit) are predicted to occur at the AM peak hour and 6 vehicle trips (4 enter/2 exit) at the PM peak hour for Stone Grey Drive East.
- A total of 4 vehicle trips (1 enter/3 exit) are predicted to occur at the AM peak hour and 6 vehicle trips (4 enter/2 exit) at the PM peak hour for Stone Grey Drive West.
- A total of 2 vehicle trips (1 enter/1 exit) are predicted to occur at the AM peak hour and 2 vehicle trips (1 enter/1 exit) at the PM peak hour for the shared driveway.
- A total of 10 vehicle trips (3 enter/7 exit) are predicted to occur at the AM peak hour and 14 vehicle trips (9 enter/5 exit) at the PM peak hour for the entire project site.
- It is recommended that two gravel shoulder widenings with drainage swales are proposed to improve the cross section of Mitchell Road and will be able to handle the minimal projected increase in vehicle trips and peak hour and all other hours.

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Proposed Development & Introduction

The proposal is to subdivide Tax Map 7, Lot 1N into fourteen single family lots. Tax Map 7, Lot 1N is proposed to contain the following: Stone Grey Drive, a 2,040 LF loop road from Mitchell Road for access to nine single family lots, Lipizzan Drive, a 664 LF cul-de-sac road accessed via Stone Grey Drive providing access to three single family lots, and a shared driveway on Mitchell Road, providing access to two single family lots. Stone Grey Drive intersections with Mitchell Road will be referred to as Stone Grey Drive East and Stone Grey Drive West. Stone Grey Drive and Lipizzan Drive are proposed to have 25-foot pavement entrance radii for emergency vehicle turning, 10-foot paved travel lanes (20 foot total paved width), and 2 foot gravel shoulders or sloped granite curbing on both sides of the roadway. Several grading and drainage improvements are proposed on Mitchell Road, including a widened gravel shoulder entrance to Stone Grey Drive East and improved roadside drainage swales between Stone Grey Drive West and the shared driveway. Off-street parking will consist of individual driveways, providing adequate parking for house lots. On street parking will be permitted in all locations except for the fire cistern apron on Stone Grey Drive and Lipizzan Drive cul-de-sac. The intersections of Stone Grey Drive and Mitchell Road and the shared driveway with Mitchell Road are the points of analysis. Stone Grey Drive East and West are located 900 feet apart. The shared driveway is located 475 feet to the west of the Stone Grey Drive West and are considered the points of analysis. The purpose of this analysis is to determine the maximum number of trips coming to and leaving Stone Grey Drive during certain peak periods of the day. This information is then used in determining the impact on safety as it relates to the existing roadway infrastructure. The following components of the analysis are typical for a project of this size pursuant to the Institute of Traffic Engineers (ITE) manual.

Existing Conditions

Existing Site Description

The existing site consists of Tax Map 7, Lot 1N containing 1,766,568 Sq. Ft. (40.55 Ac.) of land. Tax Map 7, Lot 1N is a vacant lot that is primarily wooded. Tax Map 7, Lot 1N is located in the Residential - Agricultural district. Sutton Street is located 250 feet from the eastern property corner and four residential driveway cuts exist across from the western portion of frontage on Mitchell Road.

Mitchell Road and Surrounding Roadway Descriptions

Mitchell Road is a two-lane gravel local road. This road provides access to Smoke Street/U.S. Route 4/Nottingham to the west and N.H. 125/Lee to the east. It has an Average Annual Daily Traffic (AADT) of approximately 165 (2019) divided between east and west, as shown in the traffic counts performed by Accurate Counts.



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Mitchell Road in the area of the project is composed of a gravel variable width, sixteen to twenty-foot wide gravel surface with. There is no centerline delineation and fog / edge lines provided. The posted speed limit of the roadway is 25 miles per hour (MPH). The geometry of Mitchell Road in the area of Stone Grey Drive East is relatively flat to the west (1.75% +/-), steeper to the east (4.0% +/-) and straight. The geometry of Mitchell Road in the area of Stone Grey Drive West is relatively flat to the east (1.0% +/-), steeper to the west (4.0% +/-) and is located near the apex of a curve. There are no existing sidewalks, crosswalks, or other pedestrian amenities in the area of the project.

Smoke Street & Sutton Street.

Approximately 0.2 miles to the east of the project site is the four-way, partial stop-controlled intersection of Smoke Street and Mitchell Road/Kennard Road. Kennard Road and Smoke Street are low volumes local roads. Kennard Road runs east-west and is a continuation of Mitchell Road to the West. Smoke Street runs north-south and intersects with U.S. Route 4, 0.2 miles to the north. According to traffic counts obtained from Accurate Counts, Smoke Street has an ADT 852 (2019). It is assumed that Kennard Road experiences an ADT equal to or less than Mitchell Road of 165 (2019). Due to the minimal trip generation of the proposed site and the distance from these intersections, there is no anticipated impact on the existing level of service at these intersections.

Approximately 250 feet to the east of the western front property corner of the project site is the three-way, partial stop controlled intersection of Mitchell Road and Sutton Street. Sutton Street is a local cul-de-sac that provides access to ten single family lots. There is no anticipated impact from this development on the intersection of Sutton and Mitchell Road.



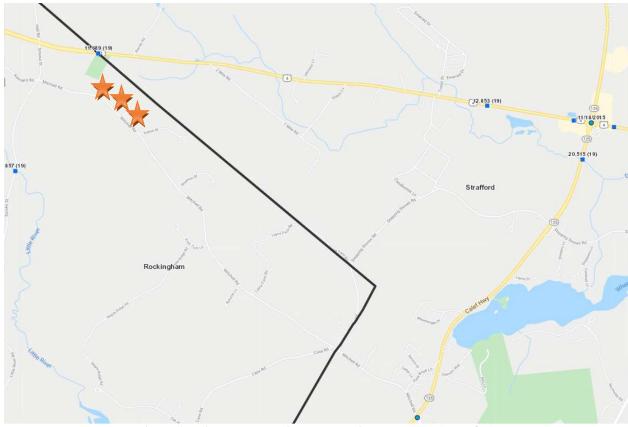


Figure 1: Mitchell Road with surrounding roadways (NHDOT)

Existing Traffic Volumes

According to traffic counts recorded by Accurate Counts for December 12 - 14, 2020, Mitchell Road experienced weekday AM, PM, and Saturday two-way peaks of 23 trips, 25 trips, and 14 respectively. It was found that Mitchell Road has an AADT of 165 vehicles.

The highest weekday peak hour traffic volume on this section of Mitchell Road eastbound occurred from 8-9 AM with 9 vehicles and from 4-5 PM with 7 vehicles. Westbound highest weekday peak hour traffic volume occurred from 7-8 AM with 17 vehicles and from 4-5 PM with 19 vehicles. Table #1 shows the traffic direction breakdown of Mitchell Road and Figures #1-3 are graphical representations of the traffic variations occurring throughout the day. It can be seen from the directional percent distribution that the primary direction of travel during the AM and PM peak hour is westbound towards Smoke Street and U.S. Route 4. Traffic counts of Mitchell Road are provided in Appendix A as Figure 7.



Date	Eastb	ound	Westboo	und	Two-Way		
Thursday	AM Peak	9	AM Peak	17	AM Peak	23	
12/12/2019	PM Peak	6	PM Peak	19	PM Peak	25	
Friday 12/13/2019	AM Peak	6	AM Peak	16	AM Peak	22	
	PM Peak	7	PM Peak	8	PM Peak	15	
Average Peak Hour	AM Peak	7.5	AM Peak	16.5	AM Peak	22.5	
Traffic	PM Peak	6.5	PM Peak	13.5	PM Peak	20.0	
% Distribution	AM Peak	31.3	AM Peak	68.8	6		
76 DISTIBUTION	PM Peak	32.5	PM Peak	67.5			
Saturday 12/13/2019	Peak	6	Peak	8	Peak	14	
% Distribution	Peak	42.9	Peak	57.1	0		

Table 1: Directional breakdown of trips occurring on Mitchell Road

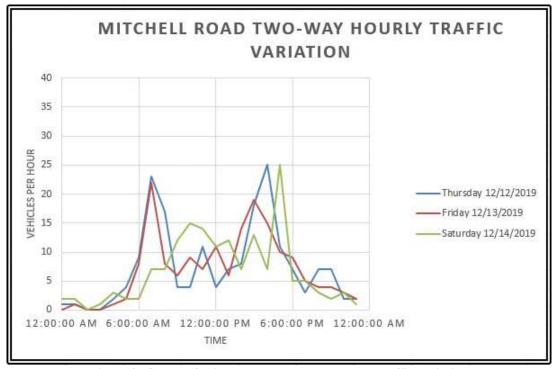


Figure 2: Graph of Mitchell Road two-way hourly traffic variation



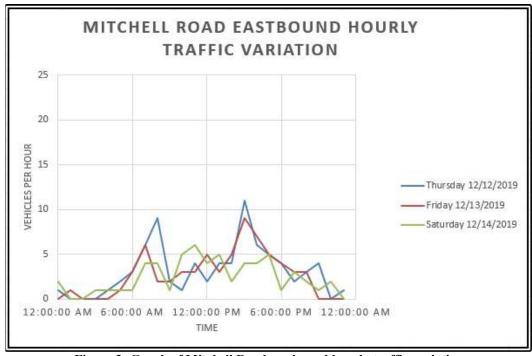


Figure 3: Graph of Mitchell Road eastbound hourly traffic variation

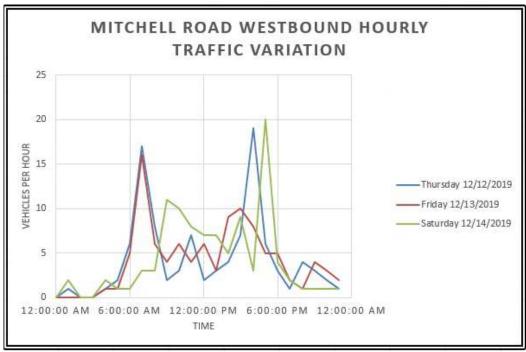


Figure 4: Graph of Mitchell Road westbound hourly traffic variation



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Existing Vehicle Speeds

As previously mentioned, the posted speed limit of Mitchell Road is 25 MPH. For the purposes of the safety analysis, the 85th percentile of speed is required. This section of Mitchell Road was measured by Accurate Counts to analyze the pass by traffic, reviewing speed. Excessive speeds were rare, and most operators obeyed the posted speed limits within a deviation of 4 MPH. The 85th percentile derived by measurement was found to be 26 MPH. Mitchell Road experienced 50th percentile speeds of 22 MPH 95th percentile speed of 29 MPH. Collected Mitchell Road vehicle speeds can be found in Appendix B as Figures #8-11.

Proposed Trip Generation

The 10th Edition ITE Trip Generation Manual was used to determine the proposed volume of trips, as well as the percentage of entrance-to-exit traffic experienced at the AM & PM peak hours between 7 and 9 AM and 4 and 6 PM, and the weekday total volume. Single Family Detached Housing (210) was used in deriving the proposed trip generation for the Stone Grey Drive and the shared driveway. Tables 2-4 provide average trip rate, total trips generated, enter to exit ratio, and the enter to exit distribution for Stone Grey Drive East, Stone Grey Drive West, and the shared driveway. Table 5 shows the combined proposed trip generation. As the use of the site will be single family residences, the primary vehicle trips generated will be two axel cars and trucks. A single-family lot was assigned either Stone Grey Drive East or West based on proximity to intersection.

Single Family Detached Housing Trip Generation Stone Grey Drive East:

Time Method	Wee	ekday Total (Pa Dwelling Units		Time Method		ak Adj. Street (i Dwelling Units		Time Method	PM Pe	ak Adj. Street (F Dwelling Units	~ .
# Units		6		# Units		6		#Units		6	
Avg. Rate		9.44		Avg. Rate	0.74		Avg. Rate		0.99		
Total Trips		56.6		Total Trips		4.4		Total Trips	5.9		
% Enter	50.0	Total Enter	28.3	% Enter	25.0	Total Enter	1.1	% Enter	63.0	Total Enter	3.7
% Exit	50.0	Total Exit	28.3	% Exit	75.0	Total Exit	3.3	% Exit	37.0	Total Exit	2.2

Table 2: (Single Family Detached) Peak hour of adjacent street traffic weekdays AM, PM, Saturday & weekday total

Single Family Detached Housing Trip Generation Stone Grey Drive West:

Time Method	Wee	ekday Total (Pa Dwelling Units		Time Method		ak Adj. Street (Dwelling Units		Time Method	PM Pe	ak Adj. Street (I Dwelling Units	~ .
# Units		6		# Units		6		# Units		6	
Avg. Rate		9.44		Avg. Rate	0.74		Avg. Rate		0.99		
Total Trips		56.6		Total Trips		4.4		Total Trips	5.9		
% Enter	50.0	Total Enter	28.3	% Enter	25.0	Total Enter	1.1	% Enter	63.0	Total Enter	3.7
% Exit	50.0	Total Exit	28.3	% Exit	75.0	Total Exit	3.3	% Exit	37.0	Total Exit	2.2

Table 3: (Single Family Detached) Peak hour of adjacent street traffic weekdays AM, PM, Saturday & weekday total



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Single Family Detached Housing Trip Generation Shared Driveway:

Time Method	Wee	ekday Total (Pag Dwelling Units		Time Method		ak Adj. Street (I Dwelling Units	· ·	Time Method	PM Pe	ak Adj. Street (f Dwelling Units	-
#Units		2		# Units		2		#Units		2	
Avg. Rate		9.44		Avg. Rate	0.74 Av		Avg. Rate	0.99			
Total Trips		18.9		Total Trips		1.5		Total Trips	2.0		
% Enter	50.0	Total Enter	9.4	% Enter	25.0	Total Enter	0.4	% Enter	63.0	Total Enter	1.2
% Exit	50.0	Total Exit	9.4	% Exit	75.0	Total Exit	1.1	% Exit	37.0	Total Exit	0.7

Table 4: (Single Family Detached) Peak hour of adjacent street traffic weekdays AM, PM, Saturday & weekday total

Total Proposed Trip Generation Stone Grey Drive and Shared Driveway

Time Method		ekday Total (Pa Dwelling Units		Time Method		ak Adj. Street (Dwelling Units		Time Method		ak Adj. Street (f Dwelling Units	~ .
Total Trips		132.2		Total Trips		10.4		Total Trips		13.9	
% Enter	50.0	Total Enter	66.1	% Enter	25.0	Total Enter	2.6	% Enter	63.0	Total Enter	8.7
% Exit	50.0	Total Exit	66.1	% Exit	75.0	Total Exit	7.8	% Exit	37.0	Total Exit	5.1

Table 5: Total combined trip generation Stone Grey Drive and Shared Driveway

Build Traffic Projections and Turning Analysis

Traffic data obtained from Accurate Counts in December 2019 has been projected to 2020 and ten years further to 2030. This has been done using a December peak seasonal adjustment factor of 2.17 (AM & PM) and using an annual growth rate of 1%, compounded annually. The derivation of the peak seasonal adjustment factor comes from an average series of values from other scenic highways from across New Hampshire, which can be found as Table 14 in Appendix D. Figures 5 and 6 show the build turning movements to and from Stone Grey Drive East, Stone Grey Drive West and a shared driveway during AM and PM peak hours. This data is used to provide a visualization of trips project to occur to and from the project site.

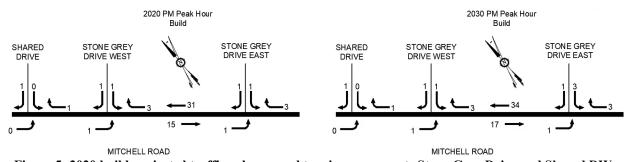


Figure 5: 2020 build projected traffic volumes and turning movements Stone Grey Drive and Shared DW



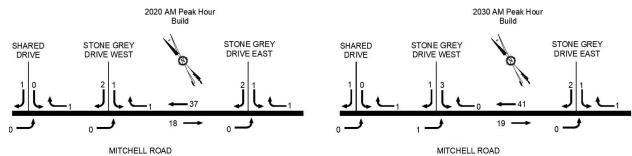


Figure 6: 2020 build projected traffic volumes and turning movements Stone Grey Drive and Shared DW

Tables 6-11 show in a tabular format the total trips that are calculated to occur to and from Stone Grey Drive East, Stone Grey Drive West, and the shared driveway are shown at AM and PM weekday peak hours in a build situation. These trips are further broken down into enter and exit to and from the site as well as percentage of left and right turns. Tables 12 and 13 show total directional breakdown of trips generated by the site.

Time	AM Peak Hour Stone Grey Drive East	#Trips	Turn Type	% Distribution
Total Trips	4.4			
Trips Er	nter from Mitchell Road Eastbound	0.3	Left	7.8
Trips En	ter from Mitchell Road Westbound	0.8	Right	17.2
Trips	Exit to Mitchell Road Eastbound	1.0	Left	23.4
Trips	Exit to Mitchell Road Westbound	2.3	Right	51.6

Table 6: Summary of AM build turning movements to and from Stone Grey Drive East

Time	PM Peak Hour Stone Grey Drive East	#Trips	Turn Type	% Distribution
Total Trips	5.9			
Trips Er	nter from Mitchell Road Eastbound	1.2	Left	20.5
Trips En	iter from Mitchell Road Westbound	2.5	Right	42.5
Trips	Exit to Mitchell Road Eastbound	0.7	Left	12.0
Trips	Exit to Mitchell Road Westbound	1.5	Right	25.0

Table 7: Summary of PM build turning movements to and from Stone Grey Drive East



Time	AM Peak Hour Stone Grey Drive West	#Trips	Turn Type	% Distribution
Total Trips	4.4			
Trips Er	nter from Mitchell Road Eastbound	0.3	Left	7.8
Trips En	ter from Mitchell Road Westbound	0.8	Right	17.2
Trips	Exit to Mitchell Road Eastbound	1.0	Left	23.4
Trips	Exit to Mitchell Road Westbound	2.3	Right	51.6

Table 8: Summary of AM build turning movements to and from Stone Grey Drive West

Time	PM Peak Hour Stone Grey Drive West	#Trips	Turn Type	% Distribution
Total Trips	5.9			
Trips E	nter from Mitchell Road Eastbound	1.2	Left	20.5
Trips Er	nter from Mitchell Road Westbound	2,5	Right	42.5
Trips	Exit to Mitchell Road Eastbound	0.7	Left	12.0
Trips	Exit to Mitchell Road Westbound	1.5	Right	25.0

Table 9: Summary of PM build turning movements to and from Stone Grey Drive West

Time AM Peak Hour Shared DW		#Trips	Turn Type	% Distribution		
Total Trips	1.5					
Trips Ente	r from Mitchell Road Eastbound	0.1	Left	7.8		
Trips Enter	from Mitchell Road Westbound	0.3	Right	17.2		
Trips Ex	it to Mitchell Road Eastbound	0.3	Left	23.4		
Trips Exi	t to Mitchell Road Westbound	0.8	Right	51.6		

Table 10: Summary of AM build turning movements to and from shared driveway

Time	PM Peak Hour Shared DW	#Trips	Turn Type	% Distribution	
Total Trips	2.0	(8)			
Trips Ente	r from Mitchell Road Eastbound	0.4	Left	20.5	
Trips Ente	r from Mitchell Road Westbound	0.8	Right	42.5	
Trips Ex	it to Mitchell Road Eastbound	0.2	Left	12.0	
Trips Exi	t to Mitchell Road Westbound	0.5	Right	25.0	

Table 11: Summary of PM build turning movements to and from shared driveway



Time	AM Peak Hour Total Generation	#Trips	Turn Type	% Distribution	
Total Trips	10.4				
Trips Ent	er from Mitchell Road Eastbound	0.8	Left	7.8	
Trips Ente	er from Mitchell Road Westbound	1.8	Right	17.2	
Trips E	xit to Mitchell Road Eastbound	2.4	Left	23.4	
Trips Ex	kit to Mitchell Road Westbound	5.3	Right	51.6	

Table 12: Summary of total AM build turning movements

Time PM Peak Hour Total Generat		#Trips	Turn Type	% Distribution	
Total Trips	13.9	E.			
Trips En	ter from Mitchell Road Eastbound	2.8	Left	20.5	
Trips Ent	er from Mitchell Road Westbound	5.9	Right	42.5	
Trips	Exit to Mitchell Road Eastbound	1.7	Left	12.0	
Trips E	xit to Mitchell Road Westbound	3.5	Right	25.0	

Table 13: Summary of total PM build turning movements

Sight Distance and Safety Analysis

Sight distance on Stone Grey Drive East and Stone Grey Drive West to the east and west, as well as roadway alignment are the two determining factors of safety. For a conservative measurement of sight distance, an 85th percentile speed of 35 MPH will be used instead of the measured 85th percentile speed of 26 MPH for Mitchell Road. For Stone Grey Drive East, sight distance to the east un-obstructed for well over 250 feet (measured), while sight distance to the west is un-obstructed for well over 250 feet (measured). Using Exhibit 3-1 (Stopping Sight Distance) (Figure 14) in the Geometric Design Manual, and a 35 mph 85th percentile speed, requires a stopping sight distance of 250 feet for eastbound and westbound traffic. To the east of Stone Grey Drive, a six foot, widened gravel shoulder on the north side of Mitchell Road is proposed for right turns entering the site. The regrading associated with this gravel shoulder allow for adequate sight distance to the east.

For Stone Grey Drive West, sight distance to the east un-obstructed for well over 250 feet (measured), while sight distance to the west is un-obstructed for well over 250 feet (measured). Using Exhibit 3-1 (Stopping Sight Distance) (Figure 14) in the Geometric Design Manual, and a 35 mph 85th percentile speed, requires a stopping sight distance of 250 feet for eastbound and westbound traffic. There are no improvements required to obtain this sight distance.

With respect to general safety of Mitchell Road in relation to the peak hour trip generation and AADT, it is our assessment that the cross section of Mitchell Road and shoulder widths require two improvements. The first improvement is the previously mentioned, six foot widened gravel shoulder on the north side of Mitchell Road to the east of Stone Grey Drive. The second



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improvement is an expanded two-foot gravel shoulder, on the north side of Mitchell Road to the west of Stone Grey Drive West. This gravel shoulder and associated drainage swale will improve the drainage conditions of Mitchell Road and help to reduce ponding water.

*AASHTO Geometric Design of Highways and Streets 7th Edition (2018)



Conclusions and Recommendations

- 1.) A total of 4 vehicle trips (1 enter/3 exit) are predicted to occur at the AM peak hour and 6 vehicle trips (4 enter/2 exit) at the PM peak hour for Stone Grey Drive East.
- 2.) A total of 4 vehicle trips (1 enter/3 exit) are predicted to occur at the AM peak hour and 6 vehicle trips (4 enter/2 exit) at the PM peak hour for Stone Grey Drive West.
- 3.) A total of 2 vehicle trips (1 enter/1 exit) are predicted to occur at the AM peak hour and 2 vehicle trips (1 enter/1 exit) at the PM peak hour for the shared driveway.
- 4.) A total of 10 vehicle trips (3 enter/7 exit) are predicted to occur at the AM peak hour and 14 vehicle trips (9 enter/5 exit) at the PM peak hour for the entire project site.
- 5.) It is recommended that two gravel shoulder widenings with drainage swales are proposed to improve the cross section of Mitchell Road and will be able to handle the minimal projected increase in vehicle trips and peak hour and all other hours.

Respectfully Submitted,

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Christopher R. Berry, SIT Principal, President

KRP/krp

Kenneth A. Berry, PE, LLS, CPSWQ, CPESC, CESSWI

Principal, VP-Technical Operations



Appendix A

Traffic Counts

Accurate Counts 978-664-2565

Location : Mitchell Road
Location : East of Smoke Street
City/State: Nottingham, NH

Start Time 12:00 AM 01:00 02:00 03:00	12/9/20 EB	WB	EB Tue	WB	Wed		Th		Fr		Sa		Sur		Week Av	
12:00 AM 01:00 02:00	•				EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
02:00				•	*	*	1	0	0	0	2	0	•	*	1	0
		*				*	0	1	1	0	0	2	*		0	1
03:00							0	0	0	0	0	0			0	0
					*		0	0	0	0	1	0		*	0	0
04:00		*	*			*	1	1	0	1	1	2		*	1	1
05:00		*	*		*		2	2	1	1	1	1	*	*	1	1
06:00							3	6	3	5	1	1			2	4
07:00		*	*				6	17	6	16	4	3	*	*	5	12
08:00					*		9	8	2	6	4	3			5	6
09:00		*			*		2	2	2	4	1	11	*		2	6
10:00		*	*		*:		1	3	3	6	5	10	*	*	3	6
11:00							4	7	3	4	6	8		*	4	6
12:00 PM	*	*	*	*			2	2	5	6	4	7	*		4	5
01:00		*	*		*:	*	4	3	3	3	5	7	*		4	4
02:00							4	4	5	9	2	5			4	6
03:00		*	*		*	*	11	7	9	10	4	9	*		8	9
04:00				*			6	19	7	8	4	3			6	10
05:00		*	*		*	*	5	6	5	5	5	10	*		5	7
06:00		*	*		*:	*	4	3	4	5	1	4	*		3	- 4
07:00							2	1	3	2	3	2		*	3	2
08:00			*		*		3	4	3	1	2	1		*	3	2
09:00	*						4	3	0	4	1	1			2	3
10:00			*				0	2	0	3	2	1	*	*	1	2
11:00	*				*		1	1	0	2	0	1		*	0	1
Lane	0	0	0	0	0	0	75	102	65	101	59	92	0	0	67	98
Day	0		0		0		177		166		151		0		165	
AM Peak	38	- 3	8	8	*	83	08:00	07:00	07:00	07:00	11:00	09:00	-	3.	07:00	07:00
Vol.	15	-	-	-	-		9	17	6	16	6	11		-	5	12
PM Peak	-	2	*	2	*	20	15:00	16:00	15:00	15:00	13:00	17:00	-	2	15:00	16:00
Vol.		-	-				11	19	9	10	5	10			8	10
0																
Comb. Total	0			0	(ř.		177	1	66		151	(0	16	i5
ADT	Α	DT 165	AA	DT 165												

Figure 7: Mitchell Road Traffic Count Summary



AADT 852

Accurate Counts 978-664-2565 Page 1 Location: Smoke Street Location: North of Mitchell Road City/State: Nottingham, NH 1849VOL2 Start 12/9/2019 Wed Thu Sun Week Average SB NB Time 12:00 AM 01:00 02:00 03:00 04:00 05:00 20 56 39 17 06:00 07:00 08:00 09:00 30 25 17 29 32 24 15 24 28 32 38 45 25 21 18 57 10 21 28 28 34 24 31 37 36 31 30 24 15 22 24 19 41 31 20 27 29 25 28 33 38 43 38 29 20 14 11 33 14 16 10:00 11:00 26 28 26 26 26 23 36 41 48 44 27 24 19 26 29 35 20 23 12:00 PM 29 22 23 25 30 29 23 01:00 26 26 34 35 23 03:00 36 51 40 35 22 16 10 26 04:00 05:00 30 06:00 07:00 08:00 23 12 15 4 20 13 11 6 12 13 09:00 11:00 409 475 Lane Day AM Peak 395 504 432 307 0 378 853 11:00 941 08:00 899 07:00 716 07:00 07:00 07:00 11:00 11:00 Vol. 32 16:00 34 14:00 12:00 26 16:00 43 Comb. 0 0 0 0 941 899 716 853 Total

Figure 8: Smoke Street Traffic Count Summary



ADT

ADT 852

Appendix B

Vehicle Speeds

							Accurate Co 978-664-25								Page 7
Location : Mitch	ell Poad						370-004-20	,00							
Location : East o															
City/State: Nottin															1849SPD1
	ignam, ivii														104931101
EB, WB															
Start	1	4	7	10	13	16	19	22	25	28	31	34	37	40	
Time	3	6	9	12	15	18	21	24	27	30	33	36	39	999	Total
12/12/19	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
01:00	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
02:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00	0	0	0	0	0	0	1	0	0	0	1	0	0	0	2
05:00	0	0	0	0	0	0	1	0	1	2	0	0	0	0	4
06:00	0	0	0	0	0	3	1	3	2	0	0	0	0	0	9
07:00	0	0	0	0	3	2	3	4	10	1	0	0	0	0	23
08:00	0	0	0	1	1	0	4	5	4	1	0	0	0	1	17
09:00	0	0	0	0	0	0	0	0	1	3	0	0	0	0	4
10:00	0	0	0	0	1	0	0	1	1	0	1	0	0	0	4
11:00	0	0	0	0	0	1	2	3	3	1	1	0	0	0	11
12 PM	0	0	0	0	0	1	1	0	1	1	0	0	0	0	4
13:00	0	0	0	0	1	1	2	1	1	11	0	0	0	0	7
14:00	0	0	0	0	1	1	3	2	1	0	0	0	0	0	8
15:00	0	0	0	0	2	3	6	3	4	0	0	0	0	0	18
16:00	0	0	0	0	1	2	8	3	5	4	2	0	0	0	25
17:00	0	0	0	0	1		1	7	0	0	1	0	0	0	11
18:00	0	0	0	0	0	1	2	1	3	0	0	0	0	0	7
19:00	0	0	0	0		2	0	0	1	0	0		0	0	3
20:00 21:00	0	0	0	0	0	0	1	3	4	1	0	0	0	0	/
22:00			0		0		1		0	0			0	0	,
23:00	0	0	0	0	4	0	2	0	0	0	0	0	0	0	2
Total	0	0	0	1	12	18	39	40	45	15	6	0	0	1	177
Total					12	- 10		40	40	- 10					
Daily		15th	Percentile :	17 MPH	4										
,		50th	Percentile :	22 MPH											
		85th	Percentile:	26 MPH	-										
		95th	Percentile:	29 MPH	4										
		Mean Speed	d(Average):	23 MPH											
			ace Speed :	18-27 MPH											
			er in Pace :	130											
			ent in Pace :	73.4%											
		er of Vehicles		52											
	Perce	ent of Vehicles	> 25 MPH:	29.4%	6										

Figure 9: Mitchell Road Vehicle Speed Summary Thursday December 12, 2019



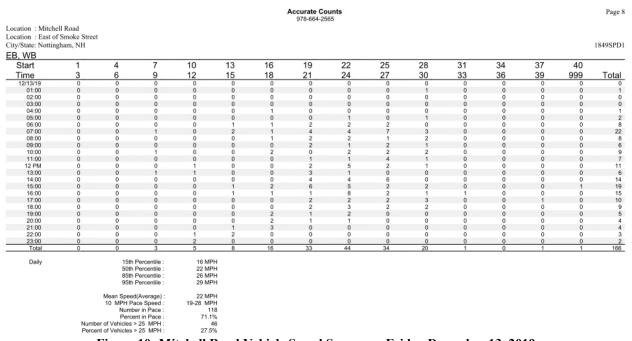


Figure 10: Mitchell Road Vehicle Speed Summary Friday December 13, 2019



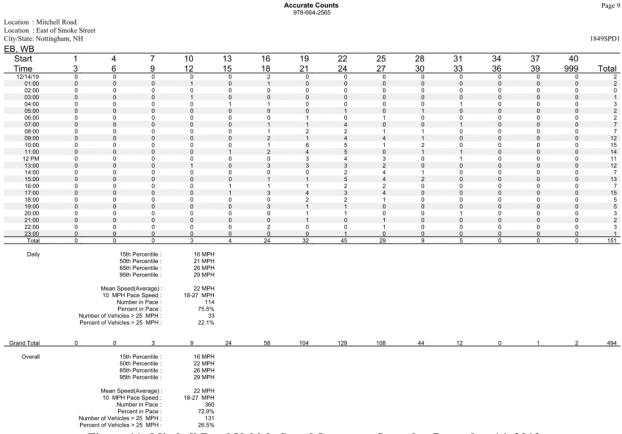


Figure 11: Mitchell Road Vehicle Speed Summary Saturday December 14, 2019



Appendix C

Trip Generation Derivation

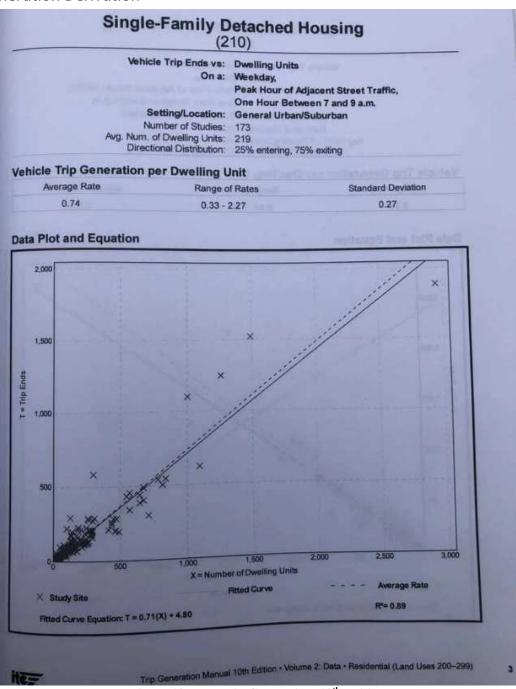


Figure 12: ITE Trip Generation, 10th Edition



BERRY SURVEYING & ENGINEERING

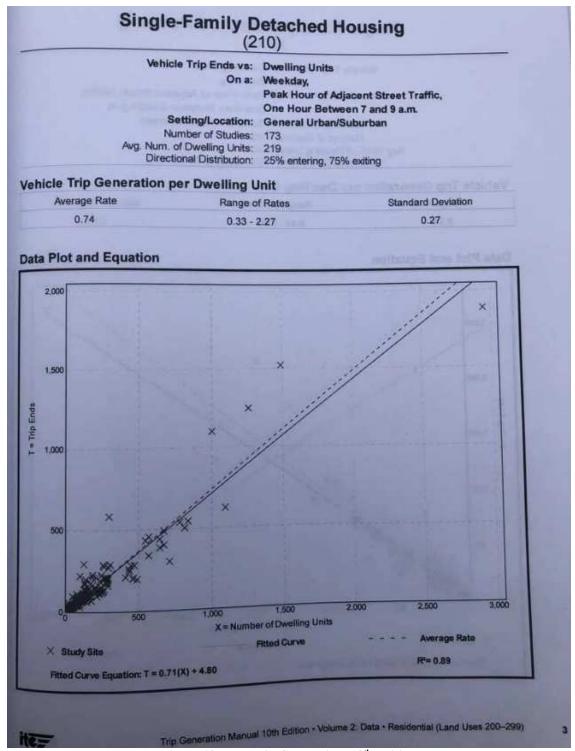


Figure 13: ITE Trip Generation, 10th Edition



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Appendix D

Miscellaneous

	Year	2018 Monthly Data				
roup 6 Averages:		Scenic Highways				
		Adjustment to				
<u>Month</u>	ADT	<u>Average</u>	<u>Peak</u>			
January	3337	1.55	2.46			
February	3961	1.30	2.07			
March	4079	1.26	2.01			
April	4230	1.22	1.94			
May	5404	0.95	1.52			
June	6851	0.75	1.20			
July	8210	0.63	1.00			
August	7831	0.66	1.05			
September	5815	0.89	1.41			
October	4944	1.04	1.66			
November	3452	1.49	2.38			
December	3787	1.36	2.17			
Average ADT:	5158					
Peak ADT:	8210					

Table 14: Derivation of the seasonal peaking factor



Table 3-1. Stopping Sight Distance on Level Roadways

	U.	S. Custor	mary	1.33.12	Exares		Metric		Sarah I
Design Speed F	Brake Reaction	Braking Distance	Stopp Sight Dis	200	Design Speed	Brake Reaction	Braking Distance	Stopping Sight Distance	
(mph)	Distance (ft)	on Level (ft)	Calculated (ft)	Design (ft)	(km/h)	Distance (m)	on Level (m)	Calculated (m)	Design (m)
15	55.1	21.6	76.7	80	20	13.9	4.6	18.5	20
20	73.5	38.4	111.9	115	30	20.9	10.3	31.2	35
25	91.9	60.0	151.9	155	40	27.8	18.4	46.2	50
30	110.3	86.4	196.7	200	50	34.8	28.7	63.5	65
35	128.6	117.6	246.2	250	60	41.7	41.3	83.0	85
40	147.0	153.6	300.6	305	70	48.7	56.2	104.9	105
45	165.4	194.4	359.8	360	80	55.6	73.4	129.0	130
50	183.8	240.0	423.8	425	90	62.6	92.9	155.5	160
55	202.1	290.3	492.4	495	100	69.5	114.7	184.2	185
60	220.5	345.5	566.0	570	110	76.5	138.8	215.3	220
65	238.9	405.5	644.4	645	120	83.4	165.2	248.6	250
70	257.3	470.3	727.6	730	130	90.4	193.8	284.2	285
75	275.6	539.9	815.5	820	140	97.3	224.8	322.1	325
80	294.0	614.3	908.3	910					
85	313.5	693.5	1007.0	1010					

Note: Brake reaction distance predicated on a time of 2.5 s; deceleration rate of 11.2 ft/s² [3.4 m/s²] used to determine calculated sight distance.

Figure 14: Derivation of stopping sight distance requirements

